

Agricultural Research Service

Channel Width Adjustment: Importance, Mechanics & Assessment

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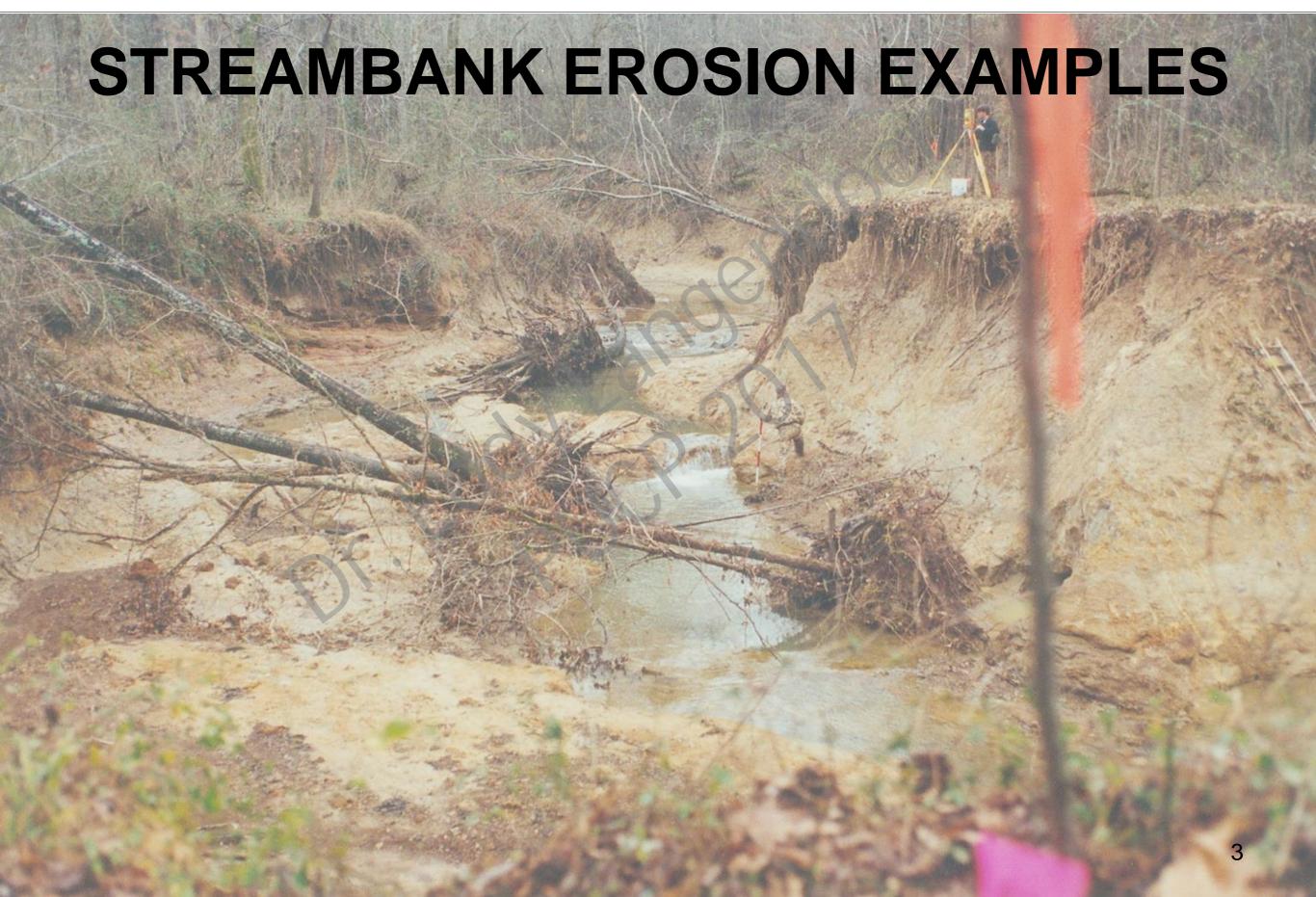
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Outline

- Examples of streambank erosion
- Channel dynamics
 - How do channels adjust?
 - Role of streambank erosion
- Streambank erosion processes & analysis
 - Fluvial erosion
 - Mass failure









WANGMO RIVER - CHINA





INDUS BASIN - PAKISTAN





INDUS BASIN - PAKISTAN



RÍO RÍMAC, PERÚ





JHIHBEN RIVER, TAIWAN – TYPHOON MORAKOT





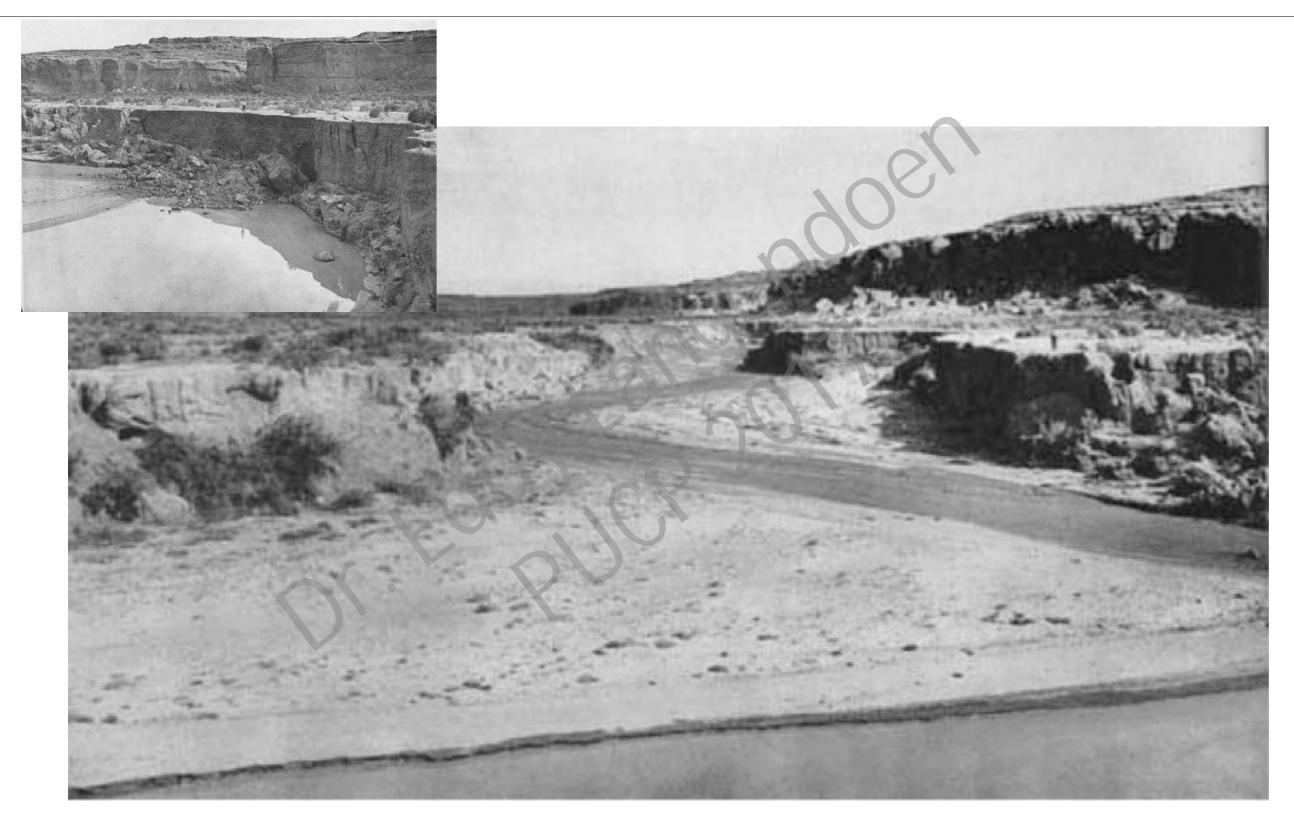
WEST PAPILLION CREEK, NEBRASKA





CANE CREEK, TENNESSEE





CHACO CANYON, NEW MEXICO





GULLY EROSION IN ETHIOPIA





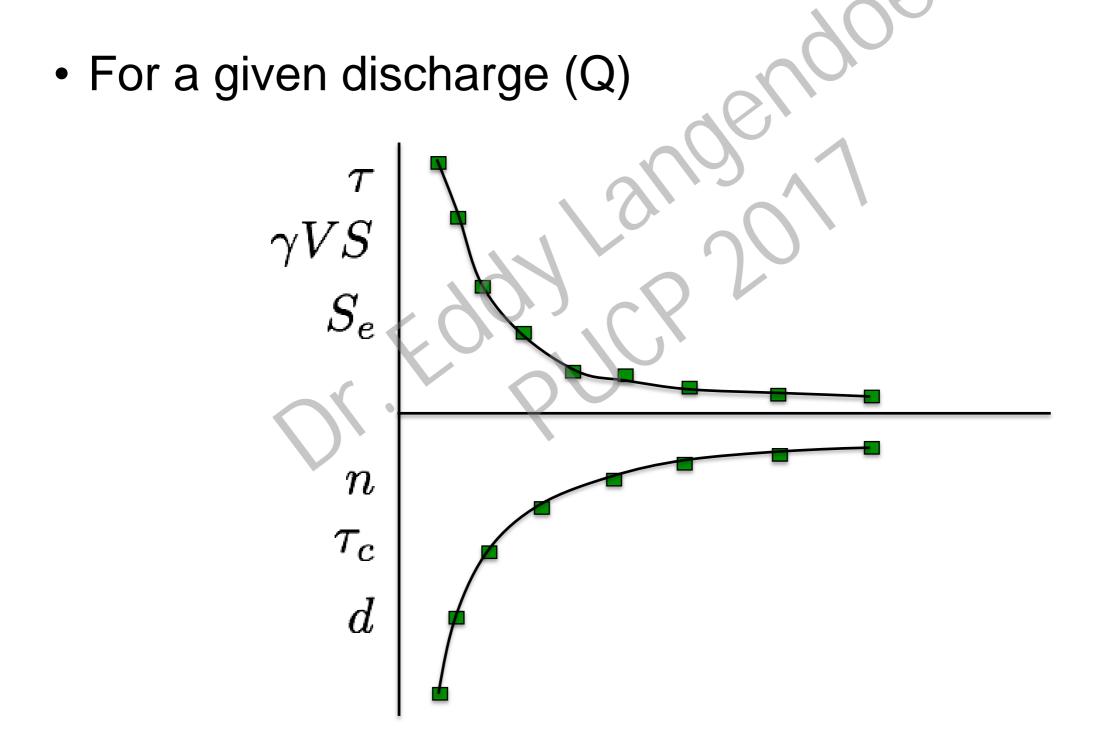


Channel Adjustment

- Longitudinal
 - Long periods of time: Headwater erosion and downstream deposition
 - Short periods of time: Local scour/deposition, degradation, and aggradation
 - Interactions between discharge, sediment supply, and slope
- Lateral, streambank erosion
 - Hydraulic: Fluvial erosion or entrainment
 - Geotechnical: Mass wasting

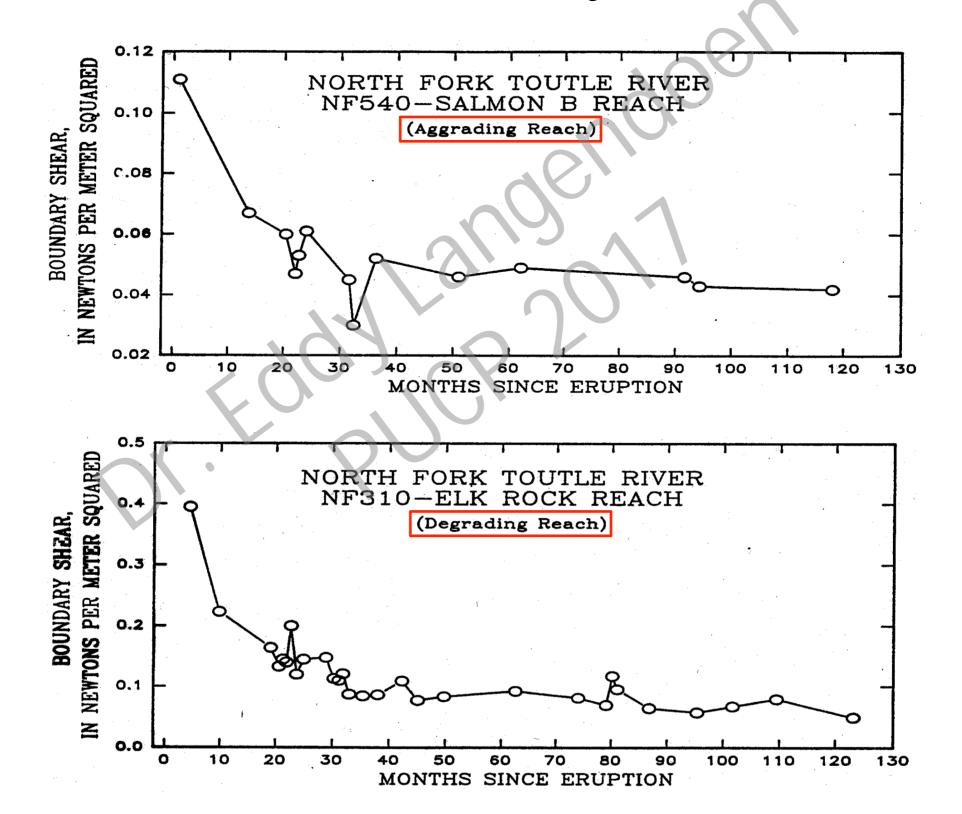


Idealized Adjustment Trends



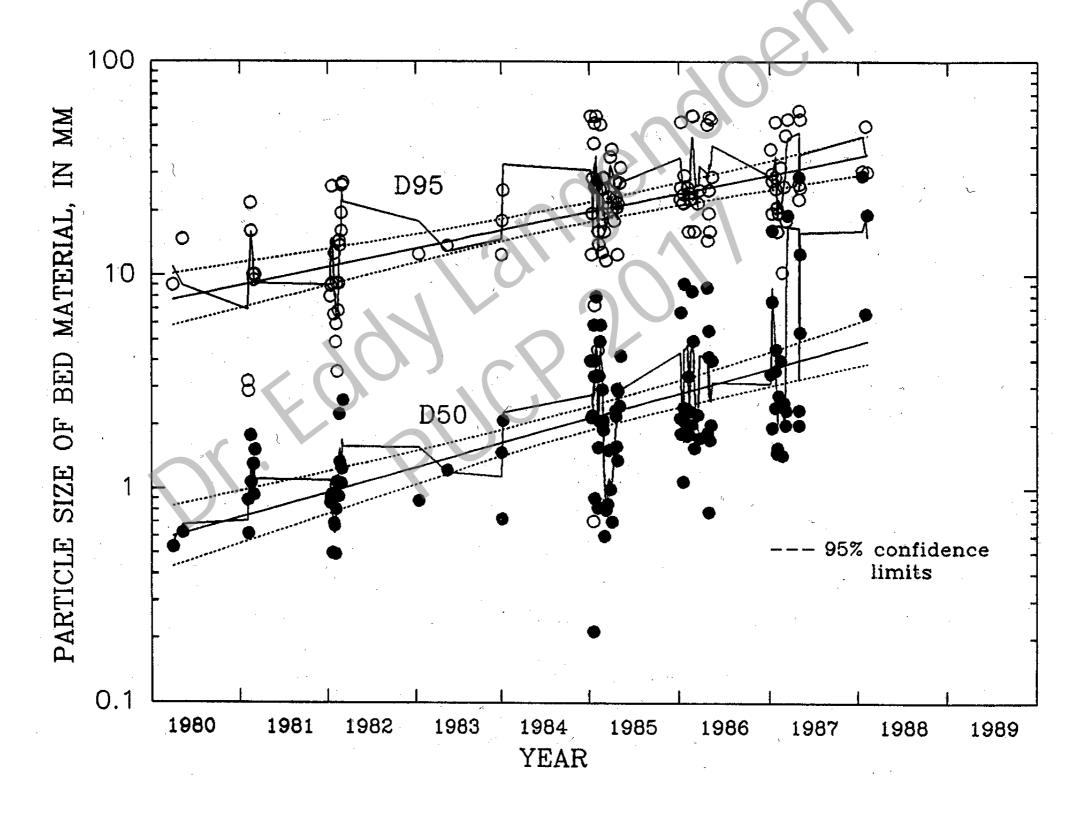


Adjustment: Boundary Shear Stress



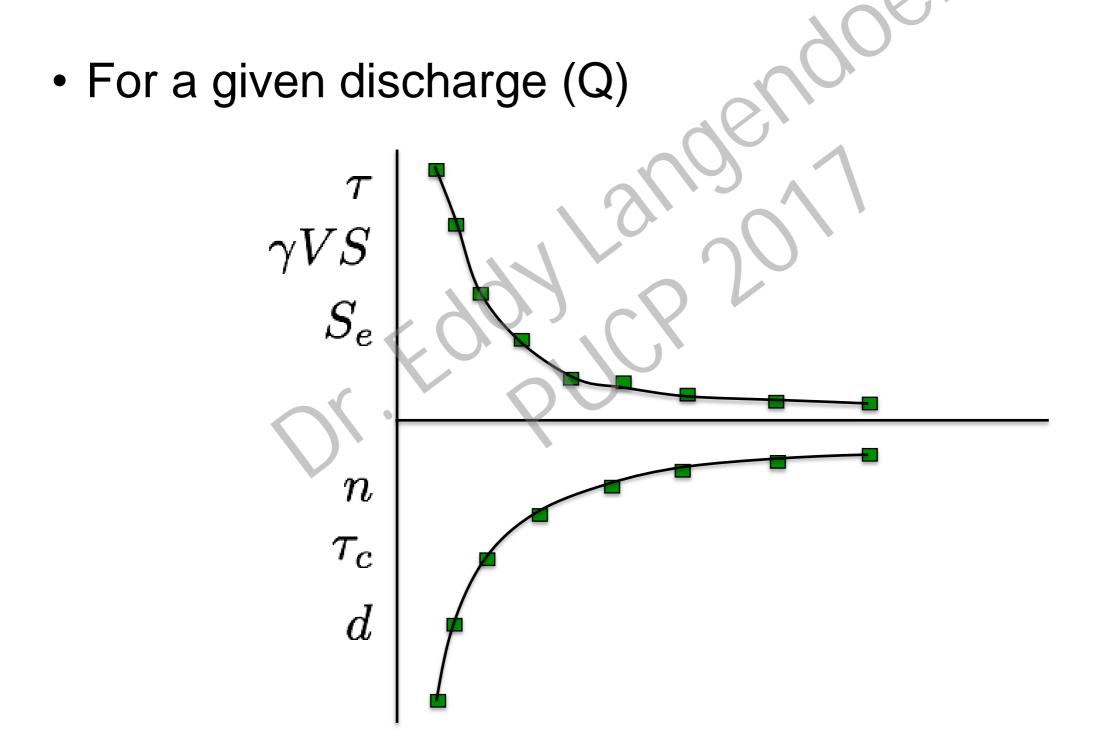


Adjustment: Increasing Resistance





Idealized Adjustment Trends



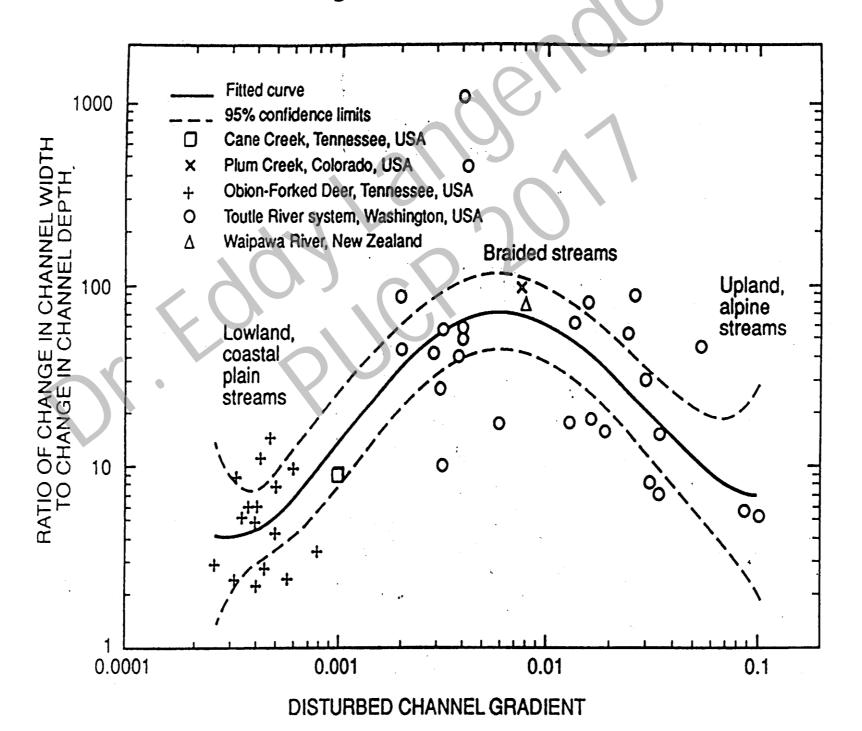


Importance of Widening in Energy Dissipation

- Reduces flow depth (pressure head) for a given flow;
- · Increases relative roughness, and therefore,
- Reduces flow velocity (kinetic energy);
- Combined with degradation (potential energy) is the most efficient means of energy reduction because all components of E are reduced;
- Counteracts increase in potential energy from aggradation



Width/Depth Changes During Adjustment





How Much Sediment Comes from Bank Failures?

- Widening rates of up to 100 m/yr
- Up to 90% of the sediment emanating from eroding channels
- Often, more than 50% of the sediment emanating from a watershed
- One 1-m failure along a 5-m high bank along a 100-m reach equals 400 metric tons or about 26 dump trucks



Little Blue River, Kansas









Streambank Erosion - I

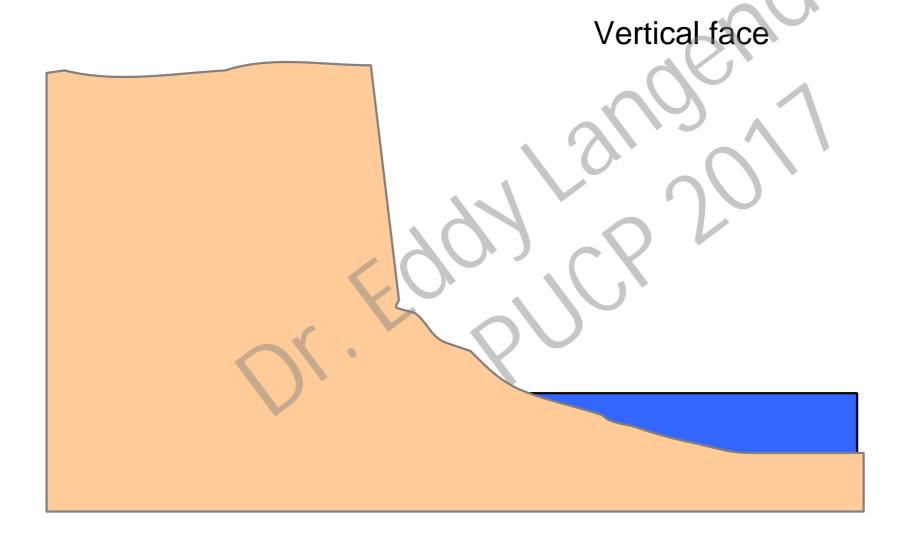
- Hydraulic processes
 - Erosive force applied by the flow exceeds the resisting force mobilized by the bank materials
 - Resisting forces are different for cohesionless and cohesive materials:
 - Cohesionless: grain size
 - Cohesive: electrochemical bonding between soil particles



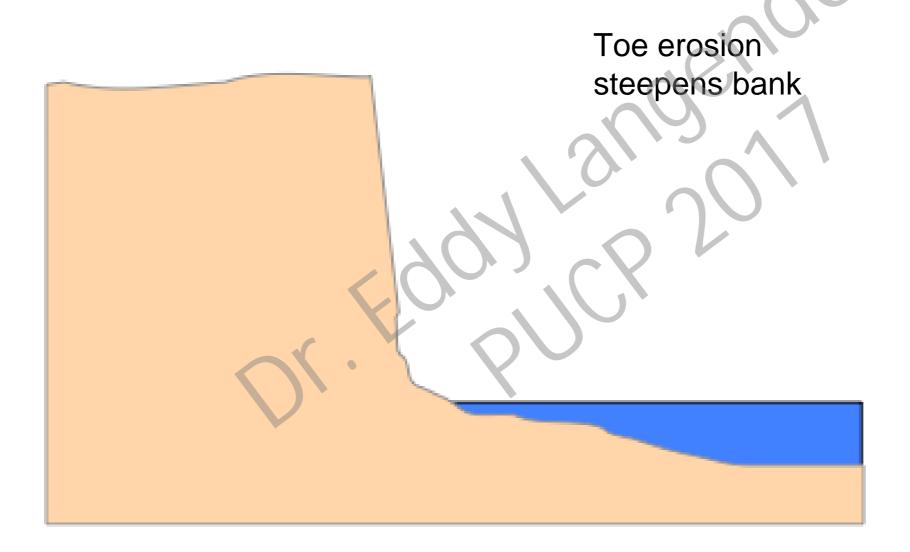
Streambank Erosion - II

- Geotechnical processes
 - Mass instability
 - Gravitational force exceed resisting force
 - · Weight of soils vs. cohesion and friction
- Interplay between fluvial erosion and mass wasting
 - Fluvial erosion may promote mass wasting
 - Mass wasting may prevent fluvial erosion

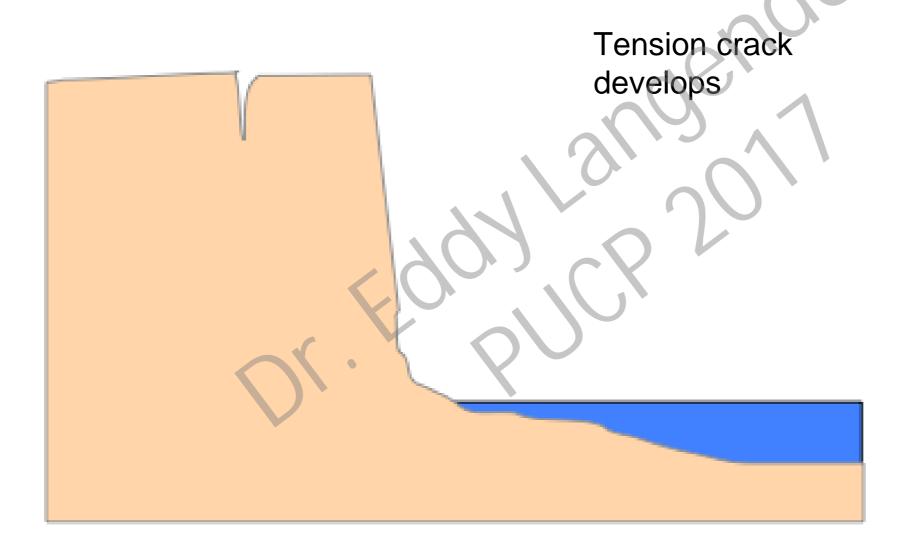




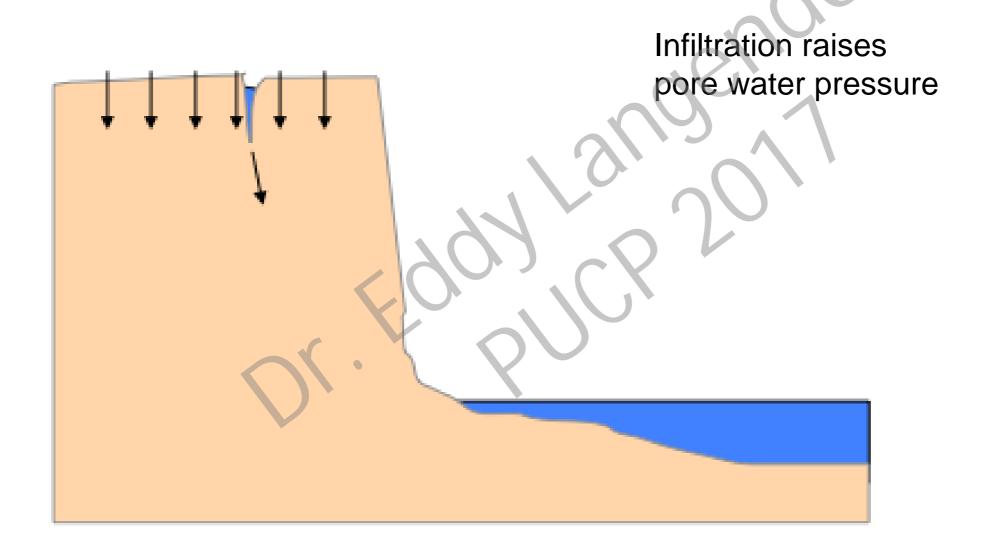




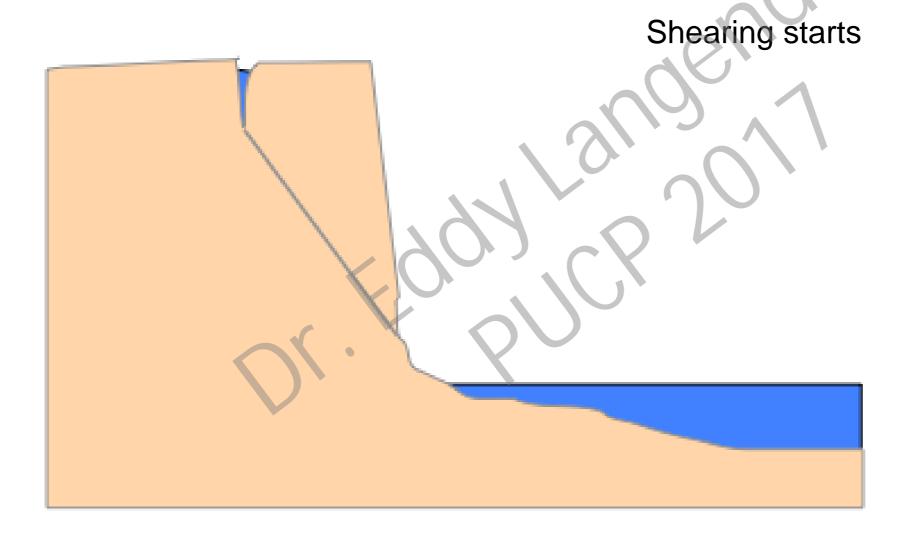




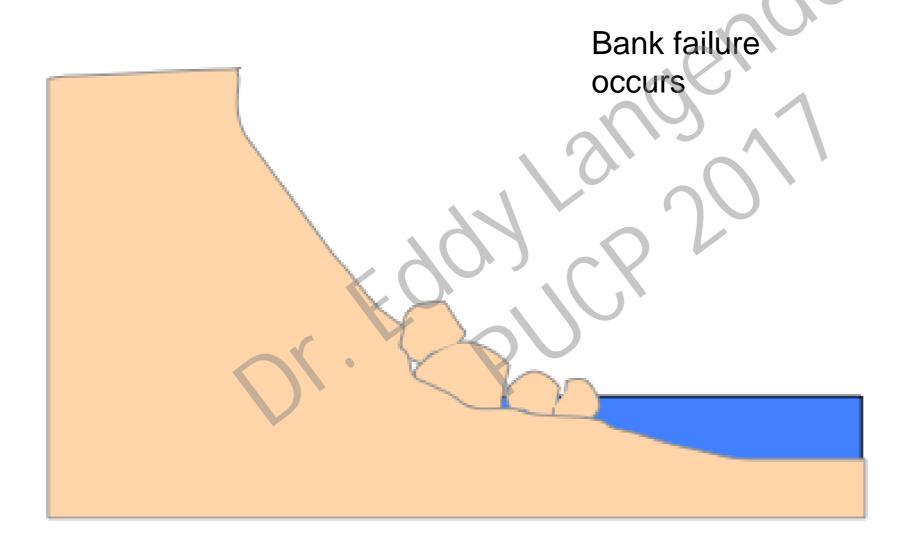




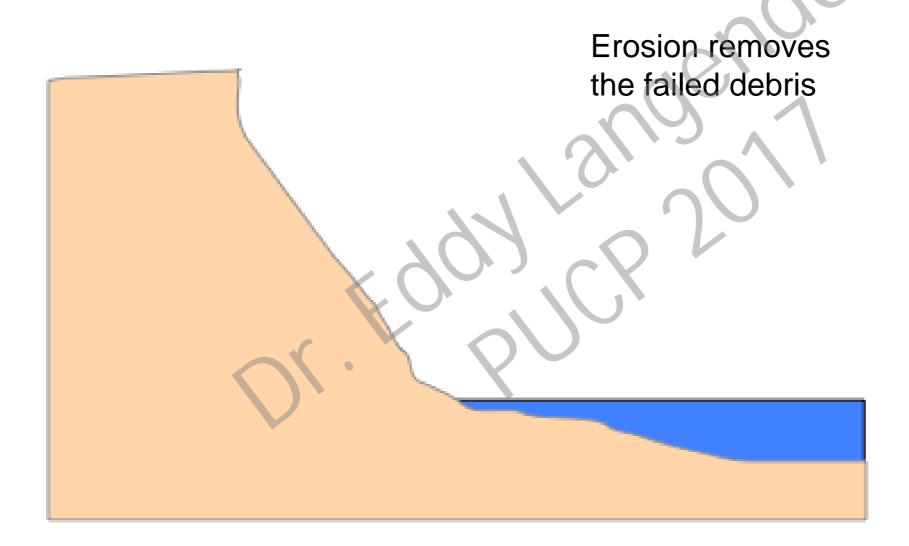




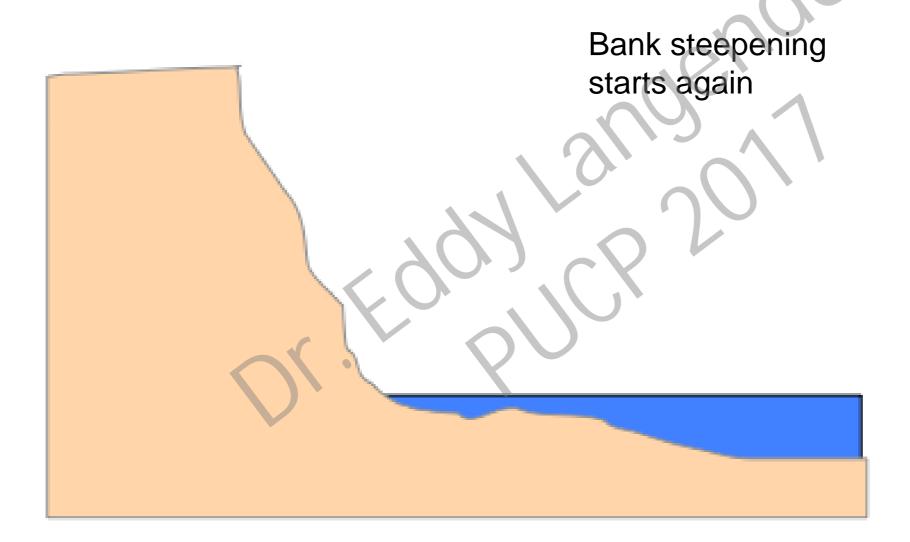




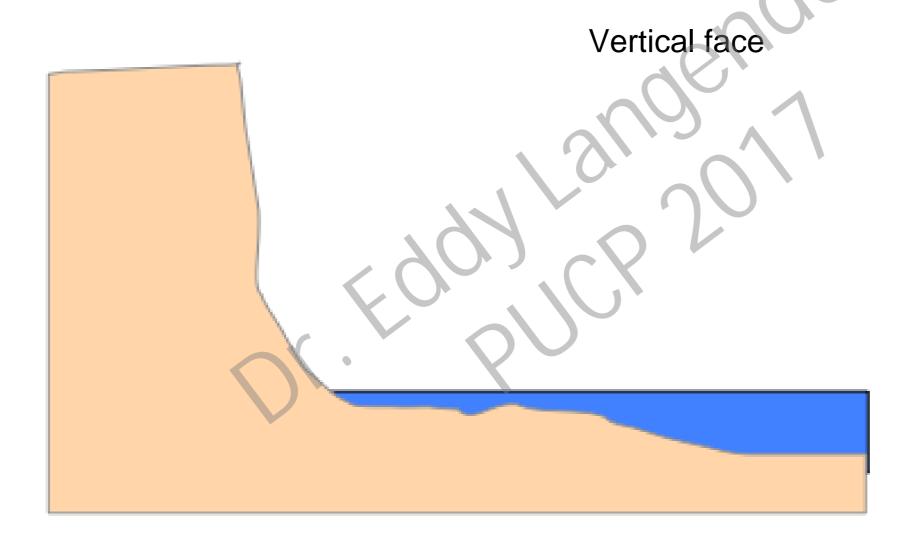






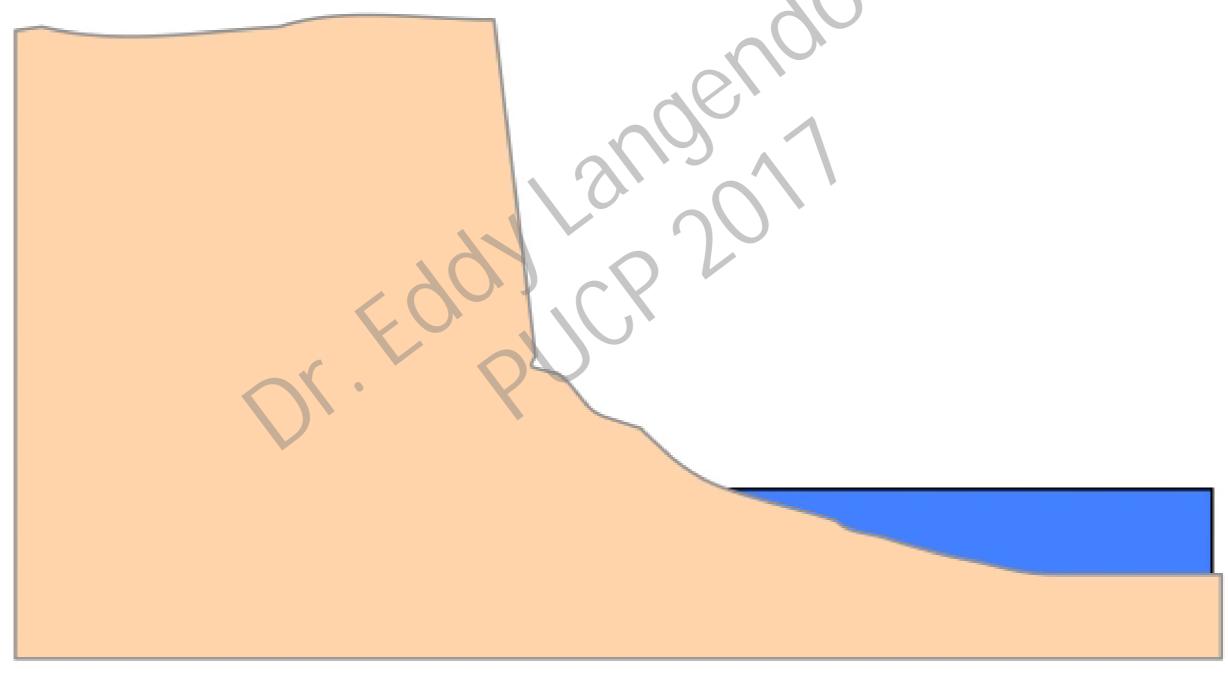








Vertical face



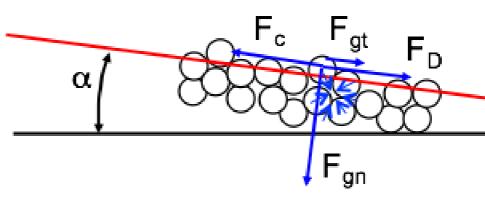






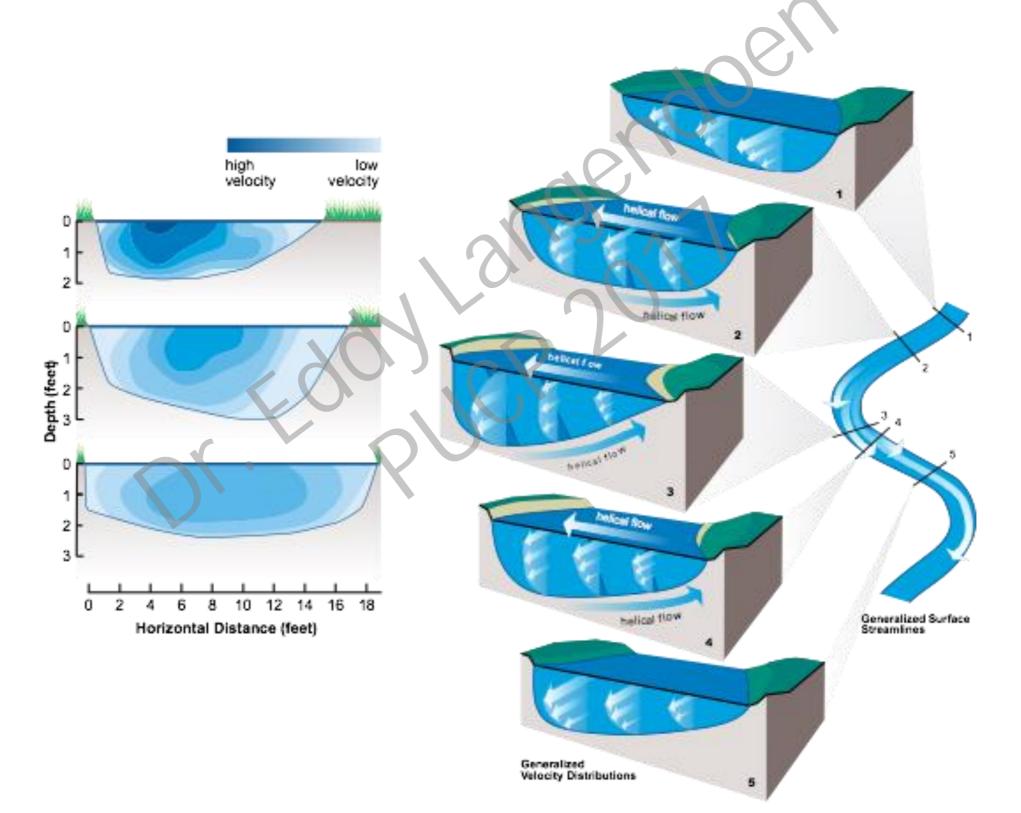
Fluvial Erosion

- Applied force vs. resisting force
- Applied drag force: boundary shear stress
 - Near-bank flow field
 - Discharge, flow depth, slope, channel form, secondary flow, turbulence, etc.
 - Bank roughness
 - Topographic variability, grain size, vegetation
- Resisting: Erosion resistance
 - Grain size
 - Cohesivity
 - Water content
 - Below-ground organics



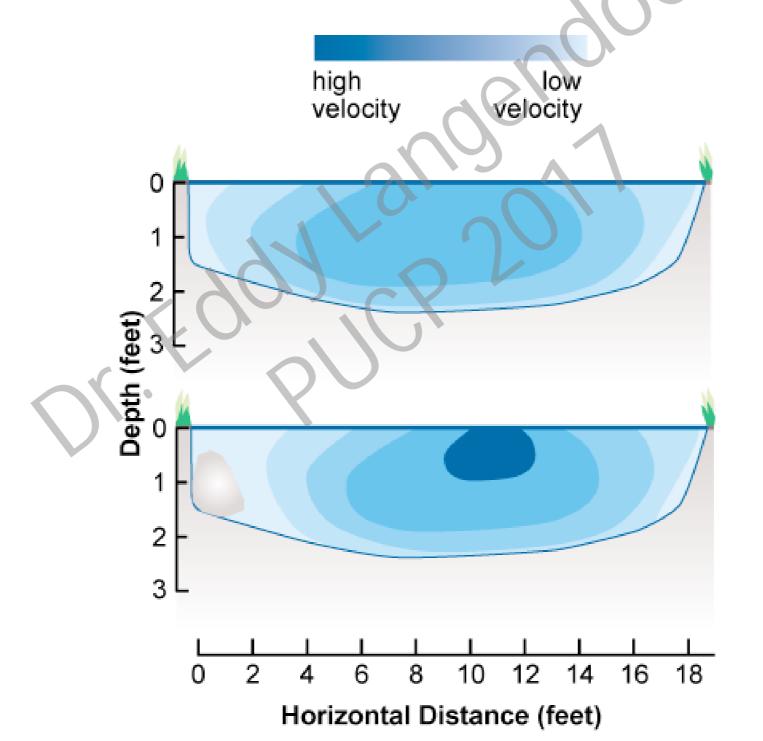


Near-Bank Flow Field





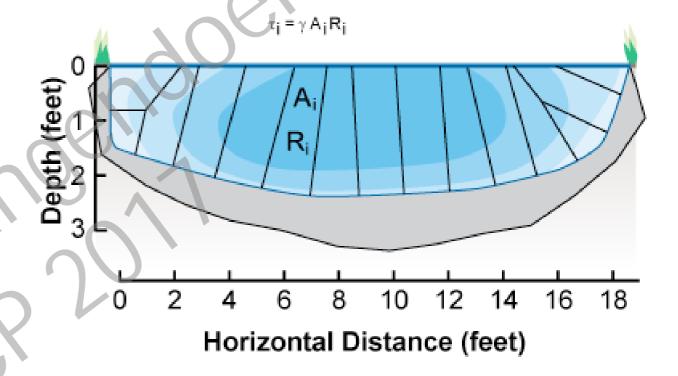
Near-Bank Flow Field /w Roughness & Cover

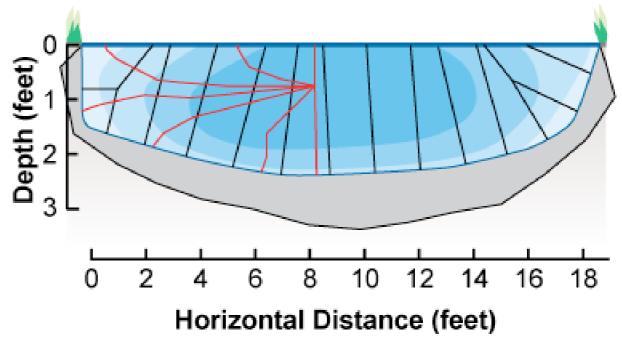




Bank Shear Stress Estimation - I

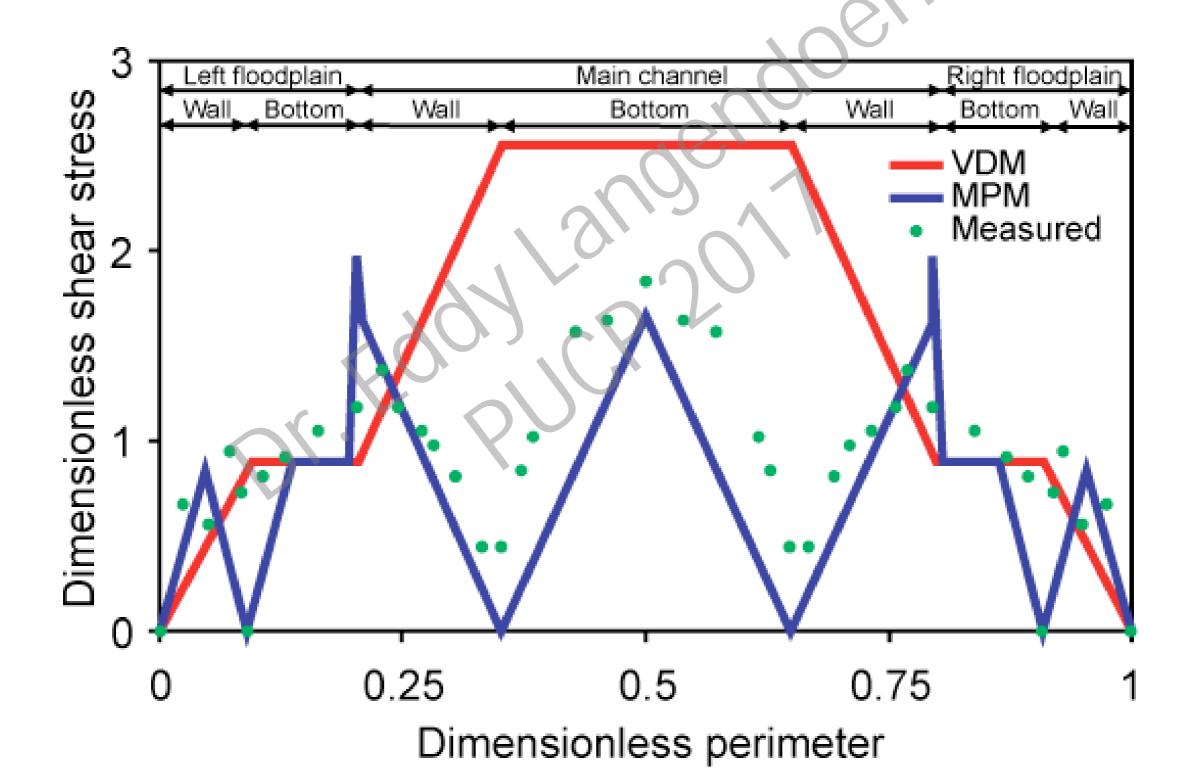
- Divided Channel Methods
 - Vertical depth/area
 - Normal depth/area
 - Merged perpendicular method
- Can have significant error for more complex flow fields







Bank Shear Stress Estimation – la

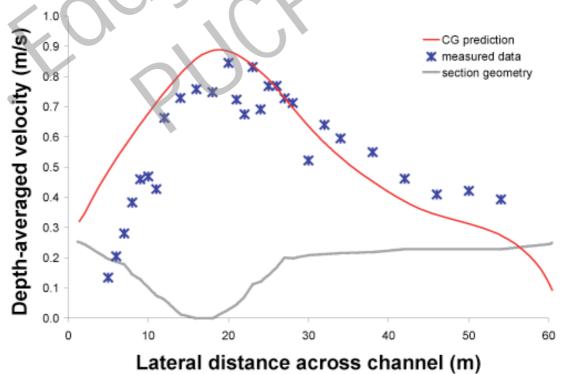




Bank Shear Stress Estimation – III

 Simplified momentum equation – Shiono & Knight method extended by Ervine

$$\underbrace{gHS}_{\text{pressure}} - \underbrace{\underbrace{\frac{f\beta q^2}{8H^2}}_{\text{boundary friction}} + \underbrace{\frac{\partial}{\partial y} \left[\lambda H \sqrt{f/8} q \frac{\partial}{\partial y} \left(\frac{q}{H}\right)\right]}_{\text{lateral shear}} = \underbrace{C_{uv} \frac{\partial}{\partial y} \left(\frac{q^2}{H}\right)}_{\text{secondary currents}}$$





Bank Shear Stress Estimation – IV

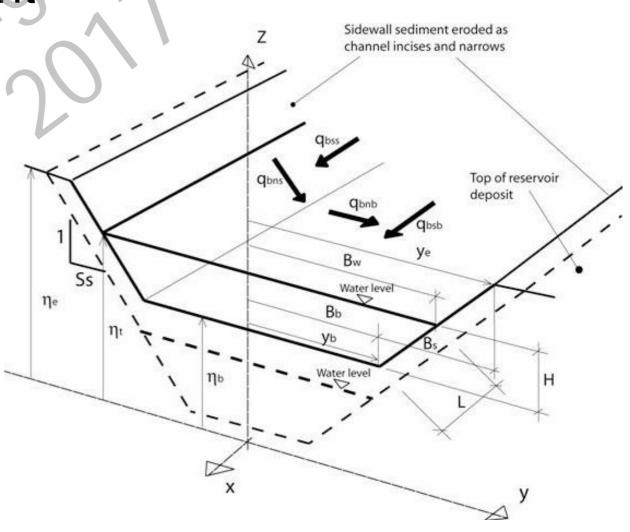
- 2D models
- 3D models



Cohesionless Bank Material Resistance & Transport

- Grain-size dependent
- Sediment transport dependent
- Cantelli and Parker (2004)

$$egin{aligned} & au_s = \phi au_b \quad (\phi = 0.4 - 0.8) \ & q_{bss} = F(au_s) \ & rac{q_{bns}}{q_{bss}} = rac{ au_{ss}}{ au_{ns}} - 2.65 \sqrt{rac{ heta_c}{ heta_s}} S_s \ & (1 - \lambda) rac{\partial \eta}{\partial t} = -rac{\partial q_{bs}}{\partial s} - rac{\partial q_{bn}}{\partial n} \end{aligned}$$





Cohesive Bank Material Resistance to Erosion

- Transport, deposition, and erosion of cohesive sediments are extremely complex
- Erosion resisting forces vary according to grain size and electrochemical bonding between particles
- Bonding is also affected by local history of soil development and antecedent soil moisture conditions
- Commonly it are aggregates that are eroded, disintegrating rapidly once entrained



Cohesive Bank Material Resistance to Erosion (cont.)

- Weathering, cycles of wetting and drying can significantly increase erodibility
- Vegetation can increase erosion resistance by
 - Binding the soil through their roots introducing an added cohesion
 - Reducing pore-water pressures in the streambank





Cohesive Streambank Erosion

Erosion rate is given by an excess shear stress relation

$$E = K \left(\tau - \tau_c \right)$$

Osman & Thorne (1988)

$$K = \frac{0.037}{\rho_s} \exp(-1.3\tau_c)$$

Hanson & Simon (2001)

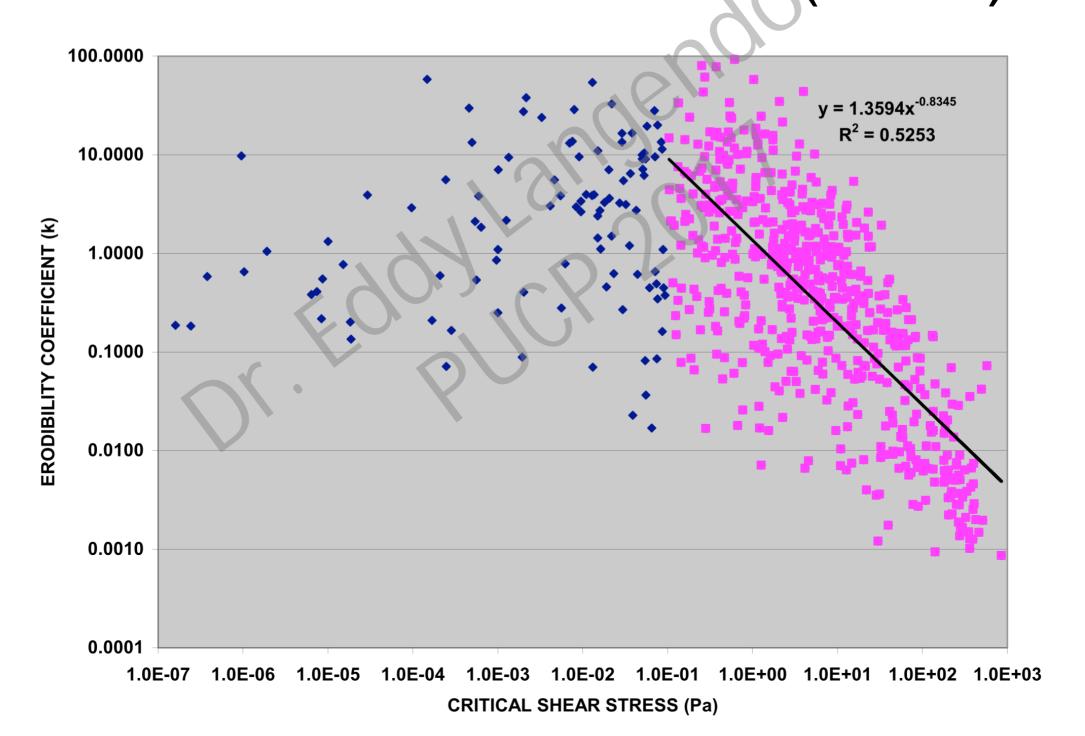
$$K = 0.1 \times 10^{-6} \tau_c^{-0.5}$$

Lateral erosion distance

$$\Delta W = E \Delta t$$

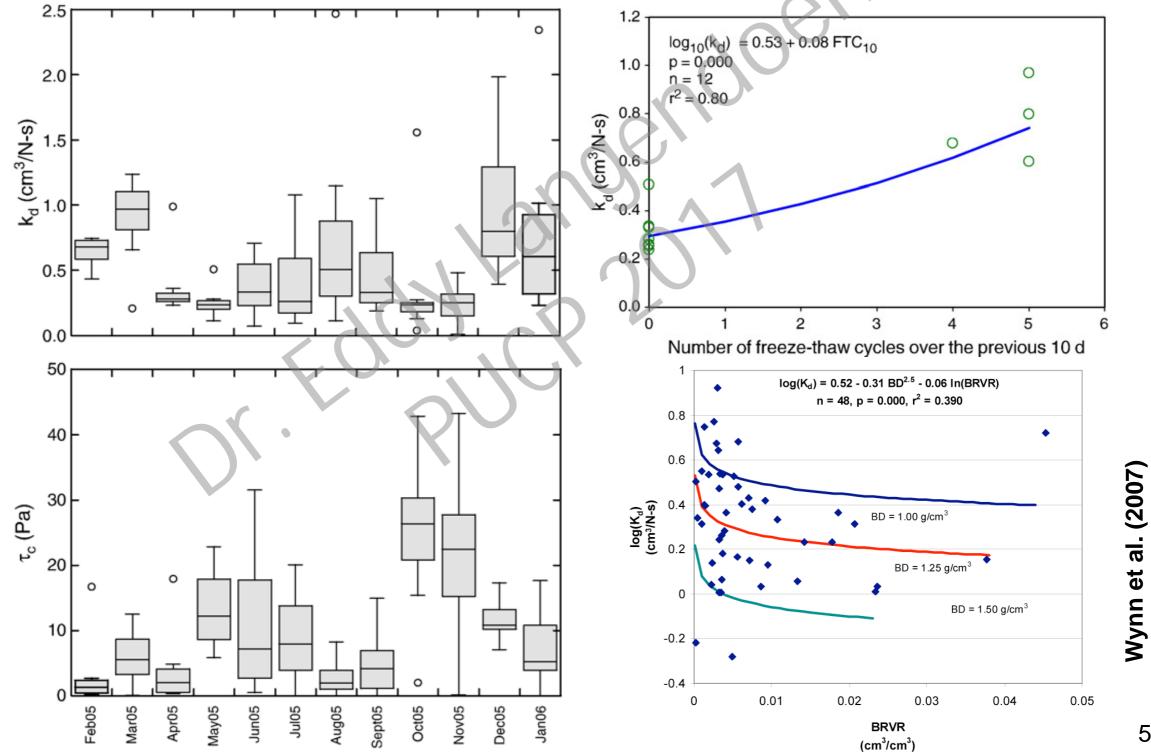


Revised Erodibility Relation – Thomas and Simon (2010)





Variability in au_c and K









Mass Failure

- Weight of the bank material exceeds the shear strength of the soil
- Often results of heightening and steepening of the bank
 - Channel incision
 - Toe erosion
- Depends on bank geometry and stratigraphy, soil water distribution, and riparian vegetation



Cohesionless vs Cohesive Mass Failures

- Cohesionless
 - Dislodgement and avalanching of individual particles
 - Shear failure along shallow slip surfaces
- Cohesive
 - Deep-seated failures
 - Block of disturbed soil sliding or toppling along a slip surface

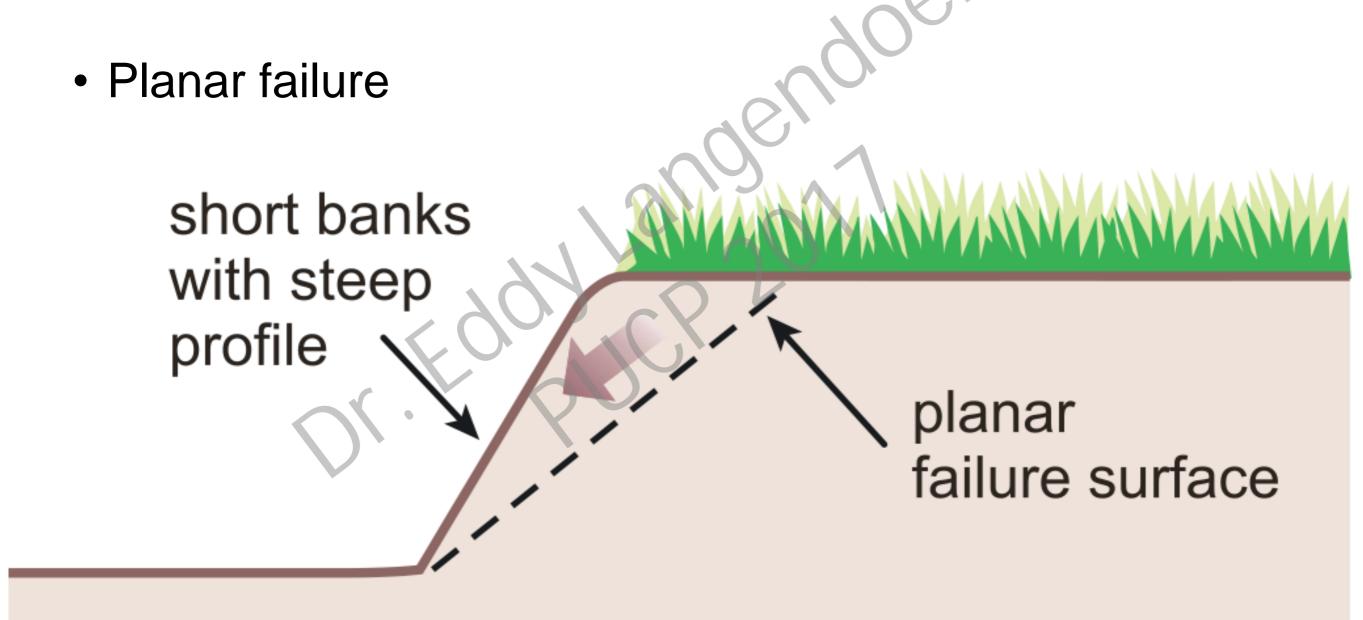


Mass Failure Mechanisms - I

 Rotational failure tall banks with shallow profile rotational failure surface



Mass Failure Mechanisms - II





Mass Failure Mechanisms - III

Cantilever failure

overhang generated on upper bank

preferential retreat of erodible basal layer

failure surface

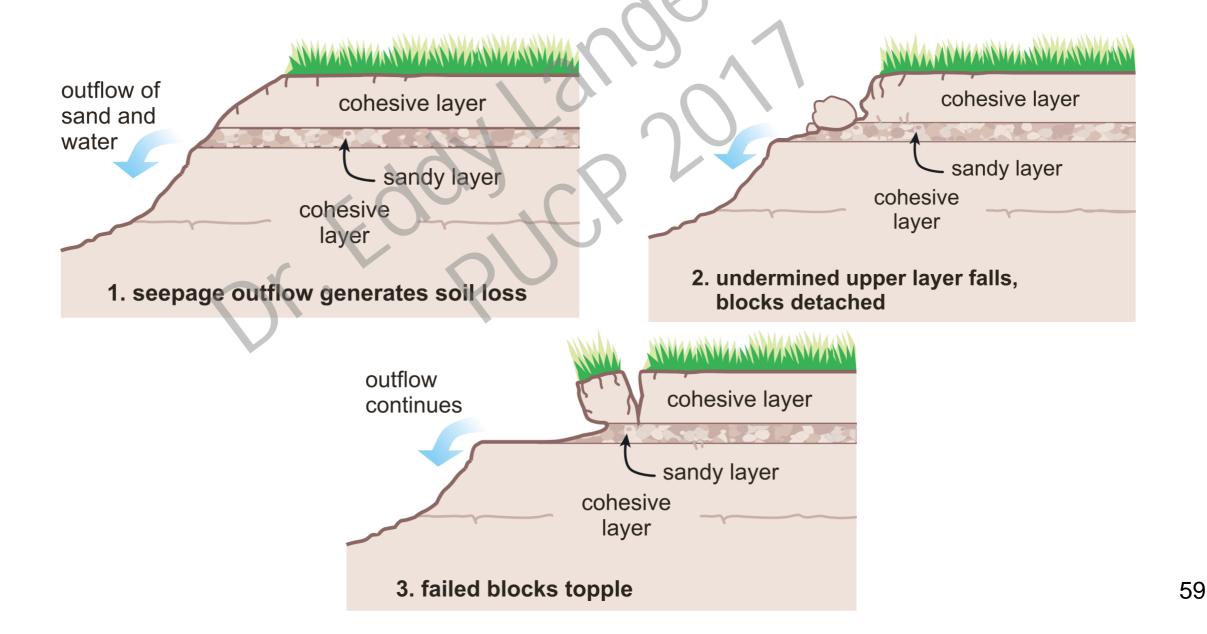
cohesive layer

noncohesive layer



Mass Failure Mechanisms – IV

Seepage or sapping failure





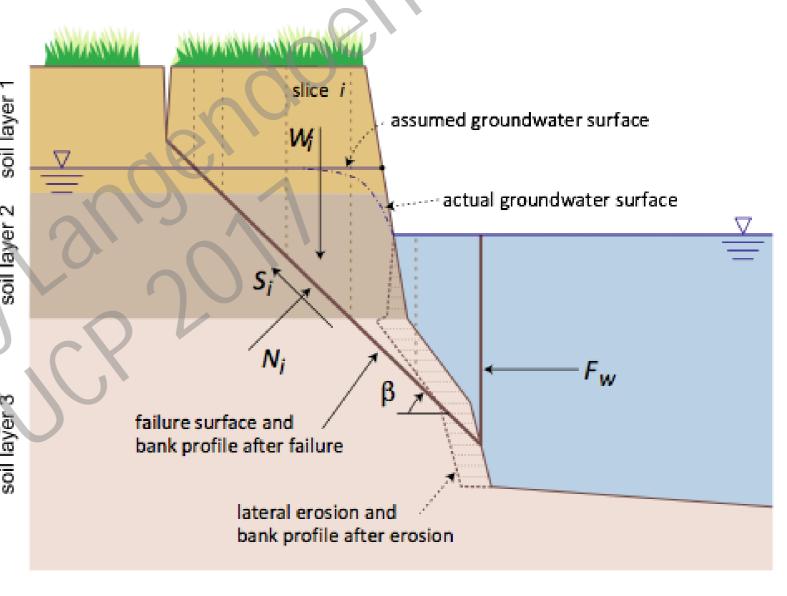
Planar Failure Analysis

- Stability charts
- Early models, such as Osman & Thorne (1988), Simon et al. (1991), Darby and Thorne (1996), had limited capabilities
 - Simplified bank profile
 - Homogeneous bank material
 - Slip surface angle prescribed
 - Evaluation of pore-water effects
- Enhancements made in late 1990s and early 2000s (e.g., Rinaldi & Casagli, 1999; Simon et al., 2000)
 - Matric suction
 - Heterogeneous bank material



Streambank Stability Analysis

- Stability is analyzed using limit equilibrium methods => FOS
 - Based upon static equilibriums of forces and/or moments
- Method of slices to account for:
 - heterogeneous bank material
 - Pore and confining pressures



 $F_w = confining force (N/m)$

N = normal force (N/m)

S = mobilized shear force (N/m)

W = weight of composite soil (N/m)

 β = angle of slip surface

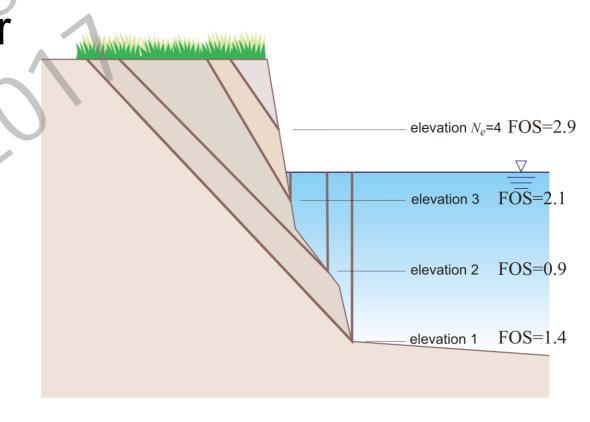


Streambank Stability Analysis – Inclination of Failure Surface

 The inclination of the failure plane is that for which the factor of safety is a minimum

$$\frac{\partial FOS}{\partial \beta} = 0$$

- CONCEPTS uses a search method to find smallest FOS
- Factor of safety is evaluated at Ne number of points along the bank profile

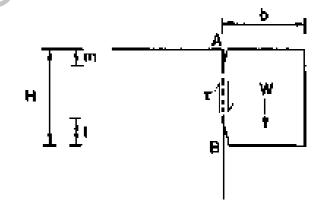




Cantilever Failure Analysis

Shear failure

$$FOS_s = \frac{(H - l - m)c}{\gamma Hb}$$

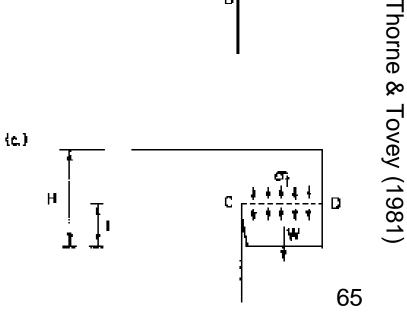


Beam failure

Tensile failure

$$ext{FOS}_b = \left(rac{H-l-m}{b}
ight)^2 rac{\sigma_t \sigma_c}{\gamma H \left(\sigma_t + \sigma_c
ight)}$$
 is

Neutral





Fate of Failure Block Material

- Failed bank material may temporally protect the bank from further eroding
- Depends on:
 - Failure type
 - Soil properties
 - Vegetation presence
 - Hydraulics & hydrology



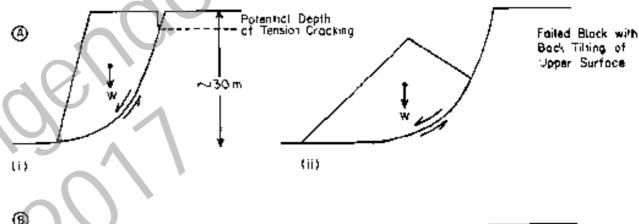


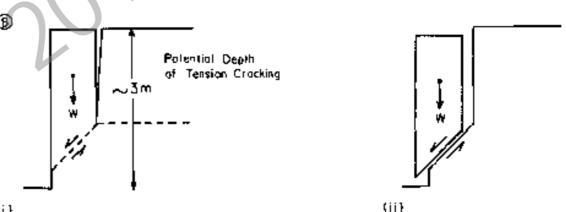


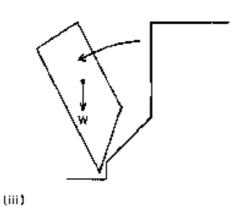
Failure Type Effects on Failure Block Fate

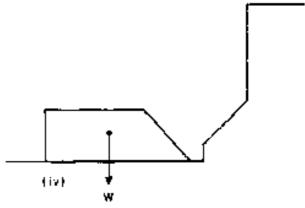
- Failure mode:
 - Sliding
 - Toppling
 - Falling

Location of failed material















Vegetation Effects on Streambank Erosion

- Reduced erosion/width:
 - Smith (1976): 20,000 times more resistance to erosion of vegetated soils
 - Beeson and Doyle (1995): erosion 30 times more prevalent on non-vegetated bends
 - Burckhardt and Todd (1998): unforested migration rate 3x
 larger
- Increased erosion/width:
 - Davies-Colley (1997): increasing width from pasture to native to forested riparian zones.
 - Trimble (1997): grassed reaches narrower than forested reaches



Vegetation Effects on Streambank Erosion (cont.)

- Resistance to surface erosion
- Resistance to failure
- Above ground biomass (stems and leaves)
- Below ground biomass (roots)
- Vegetation affects erosion through:
 - Raindrop interception
 - Increased infiltration and infiltration capacity
 - Soil water transpiration
 - Increased surface roughness
 - Soil aggregate stability
 - Soil reinforcement



Vegetation Effects on Streambank Stability

Shear strength equation

$$au = c' + \sigma_n an \phi' - p an \phi^b$$

- Shear strength increases with increasing cohesion and decreasing pore-water pressure
- Vegetation affects both cohesion (i.e., soil bonding) and soil water content



Vegetation Effects on Streambank Stability (cont.)

	Mechanical	461	Hydrologic
Stabilizing	Increased strength due to roots	Transpi	ration and canopy interception
Destabilizing	Surcharge	Increas	ed infiltration rate and capacity



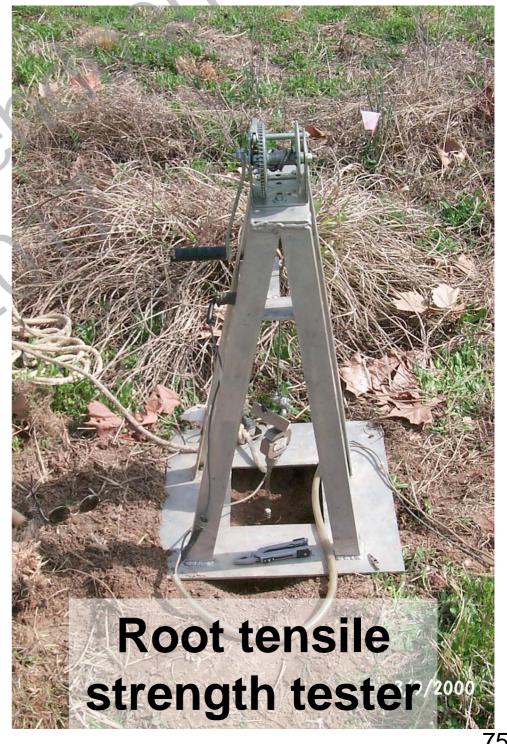


Mechanical Effects of Vegetation

'Root' cohesion

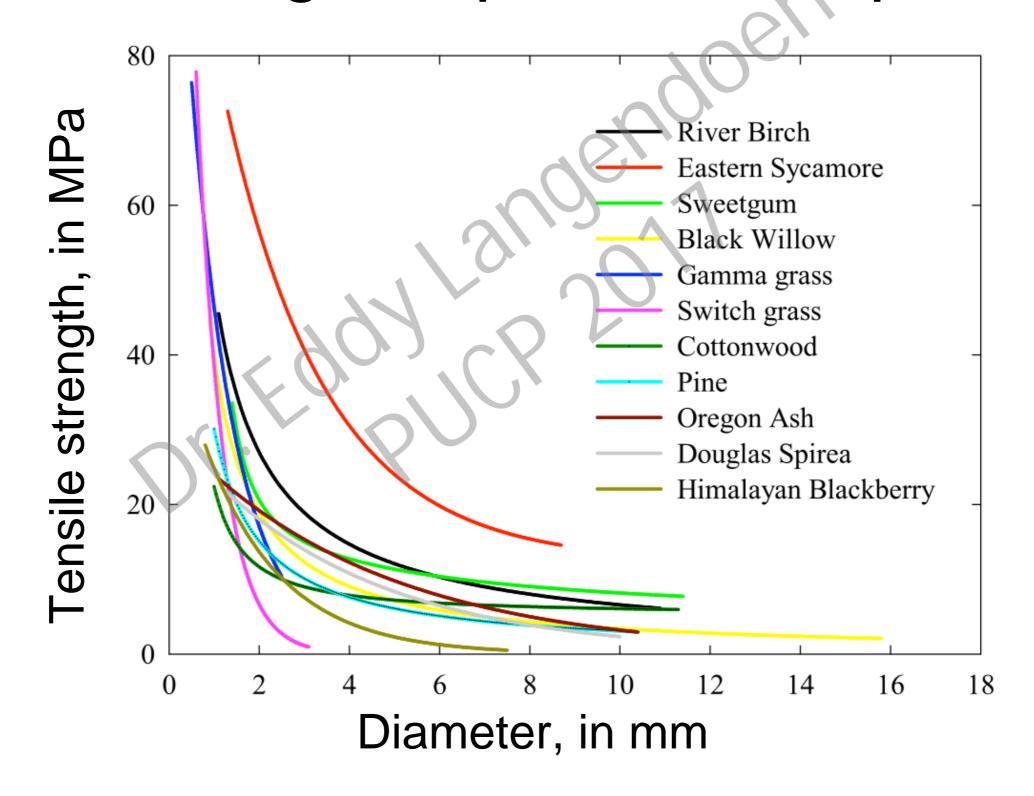
$$c_r = 1.2T_r \left(A_r / A \right)$$

- Two parameters:
 - Root tensile strength
 - Root-Area-Ratio
- Species-dependent





Root-Strength: Species Comparison





Root Distribution



Trench around suitable riparian trees, and date tree using dendochronology

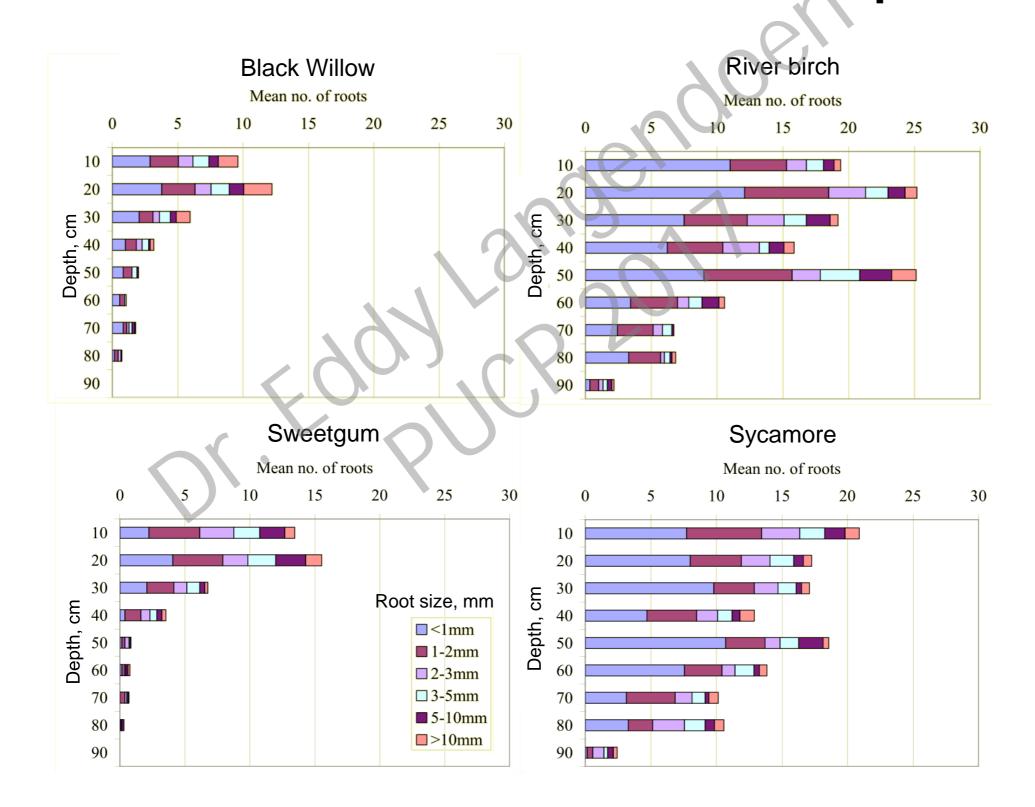
Map roots on frame and measure diameter with calipers





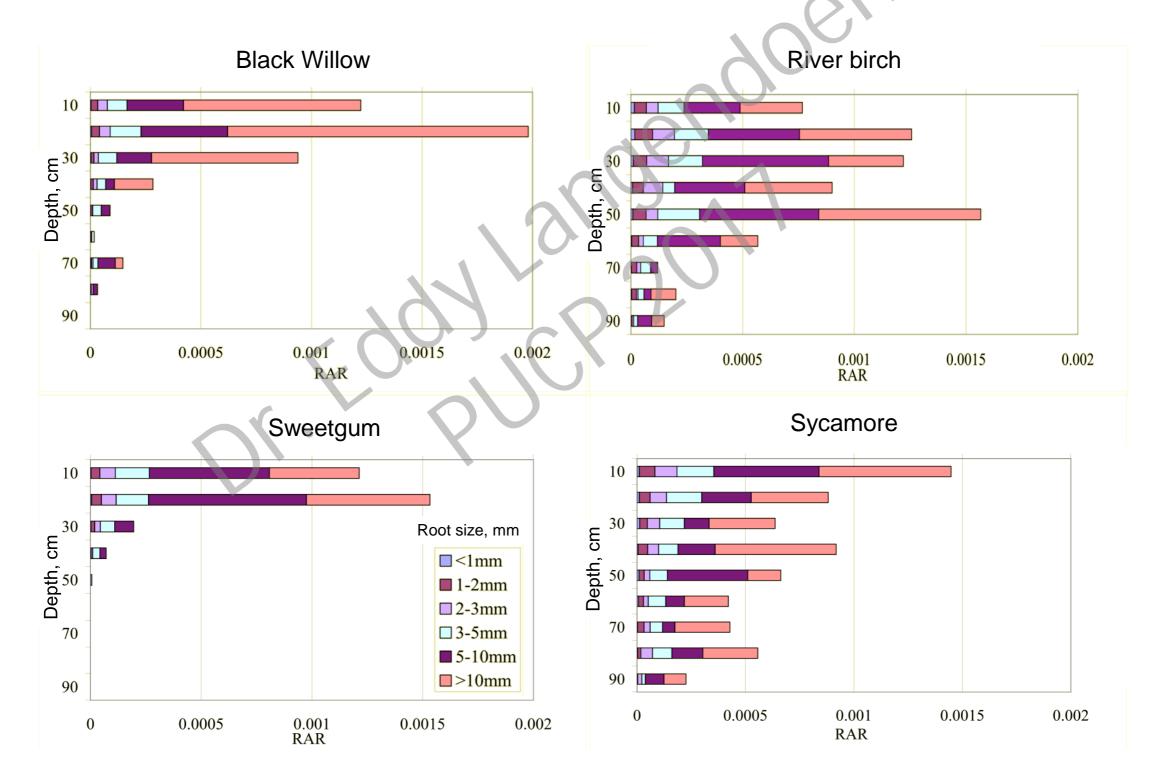


Number of Roots vs Depth



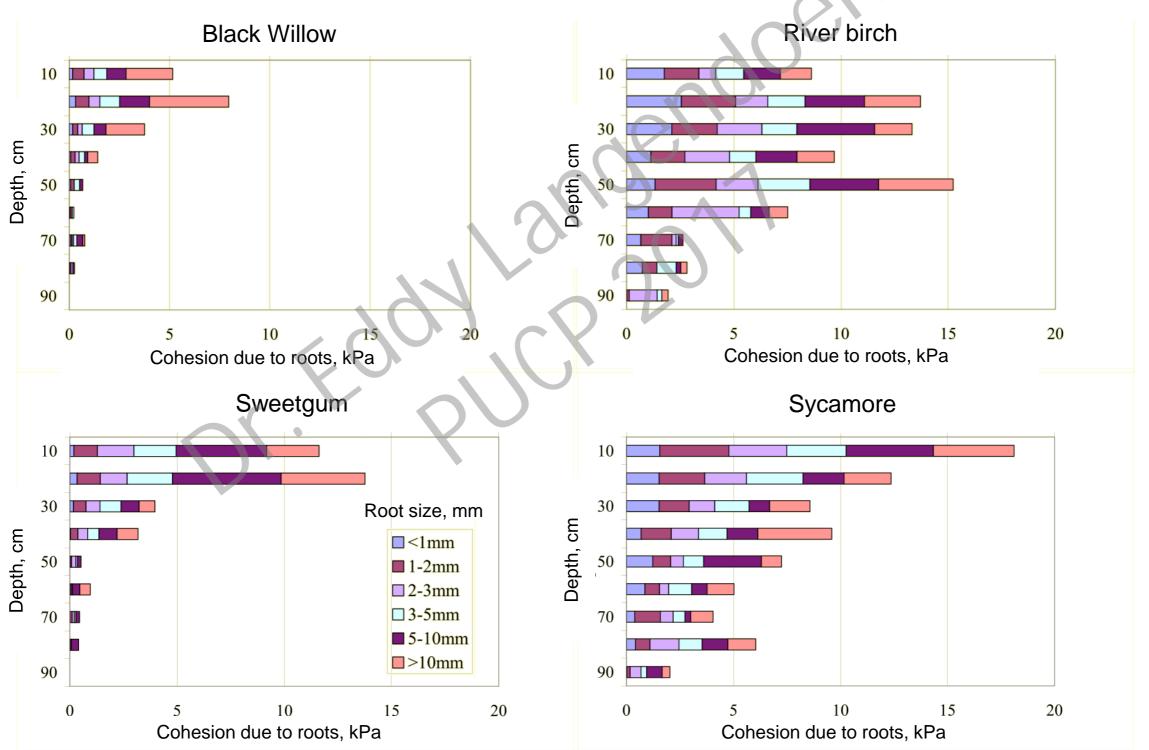


Root Area Ratio vs Depth





Cohesion Due to Roots vs Depth





Mechanical Findings

- Trees add 5-20 kPa cohesion to soil, over about 0-100 cm depth (black willow least effective)
- Clump grasses add 10-40 KPa cohesion
- Lots of small roots potentially provide greater strength than a few big roots
- However most of the strength from trees actually comes from large sized roots – small roots make up too little area
- Significant strength achieved over 5-10 years growth



Quantifying the Hydrologic Effects of Vegetation

- Beneficial Effects:
 Increase in strength due to matric suction and reduced pore-water pressure
 - Rainfall Interception
 - Transpiration
- Detrimental Effects:
 - Enhanced infiltration
 - Enhanced permeability





Monitoring the Hydrologic Effects of Vegetation

- Rainfall, stemflow and throughfall monitored spatially and in real-time
- Pore-water pressure below plots monitored using tensiometers



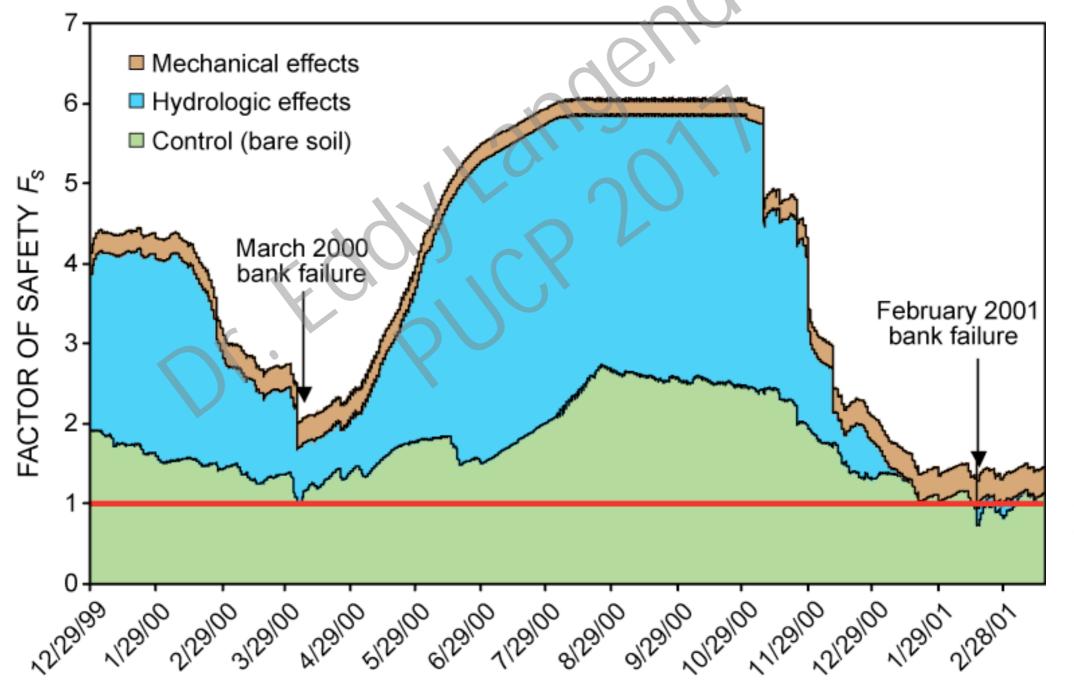


Hydrology Findings

- 2% of rain is intercepted by riparian strip canopy (high intensity events, low canopy cover during winter/spring)
- Trees increase infiltration capacity, concentrating more water in upper 30-100 cm soil than on bare or grasscovered banks
- Trees maintain suction at depth (200-300 cm) into spring
- High matric suction at depth indicates deeper roots than found in survey (?)



Vegetation Effects on Streambank Stability (cont.)



From Simon and Collison, ESPL, 2002



Assessment Tools

- Single bank
 - BSTEM
- 1D models
 - CONCEPTS
 - HEC-RAS
- 2D models
 - SRH-2D
 - Telemac2D/Sisyphe
 - RVR-Meander







Streambank Erosion Input Data Requirements

- Bank material properties
 - Composition
 - Unit weight
 - Erodibility
 - Shear strength
- Bank roughness
- Bank stability analysis options



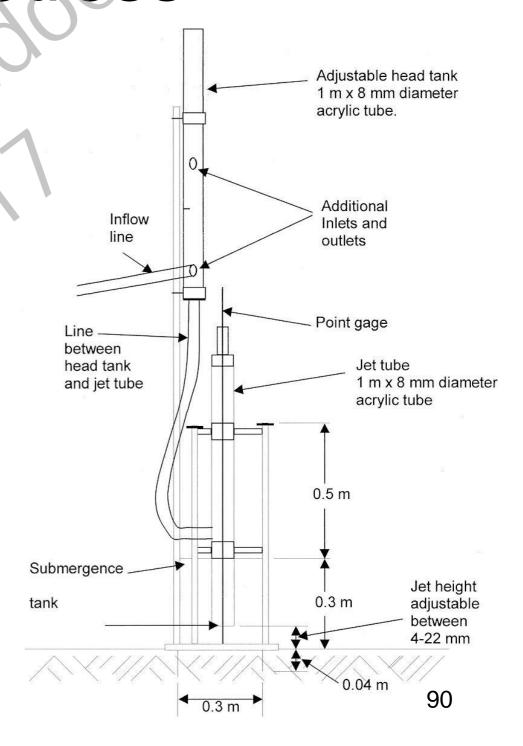
Flume Methods to Determine Critical Shear Stress

Device	Known flow conditions	Field tests	Bedload	Depth range	Armoring	Erosion rate	Shear stress range
Annular flume	Yes	No	No	0-0.2 m	Yes	No	0-1 Pa
SEDFlume	Yes	Yes	No	0-1 m	No	Yes	0-10 Pa
SEDFlume /w trap channel	Yes	No	Yes	0-1 m	No	Yes	0-10 Pa
Oscillatory flume	Yes	Yes	No	0-1 m	Some	Yes	0-10 Pa



In-Situ Jet Test Device to Determine Critical Shear Stress

- Developed by the Agricultural Research Service (Hanson, 1990).
- Based on knowledge of hydraulic characteristics of a submerged jet and the characteristics of soil-material erodibility.
- Apparatus: pump, adjustable head tank, jet submergence tank, jet nozzle, delivery tube, and point gage.
- The stress range = 4 1500 Pa.
- Maximum scour measurements are taken at five to ten minute intervals over a period of 60 to 120 minutes.





Closing remarks

- Bank erosion and channel width adjustment is common
- Channel width adjustment can be orders of magnitude greater than channel depth adjustment
- Bank erosion is controlled by fluvial and gravitational processes
- Bank erosion assessment is complex because of spatial variations in soil texture and the erosion mechanics of fine-grained soils